ROCKFORD BOARD OF EDUCATION
INVITATION FOR BID ON SUPPLIES, MATERIALS, EQUIPMENT OR SERVICES FOR SCHOOL DISTRICT NO. 205 ROCKFORD, ILLINOIS

IFB No. 22-40 East H.S. Student Parking Lot Improvements

DATE: April 13, 2022

RE: **ADDENDUM NO. 1**

To All Bidders:

Included are modifications, clarifications and/or corrections for the Project Manual and are hereby made a part of the contract documents. Please attach this addendum to the Project Manual(s) in your possession. Please note the receipt of this addendum on the bid form. Bidders shall review changes to all portions of this work as changes to one portion may affect the work of another.

If you plan to hand deliver your IFB submission on the due date, please note you must check in on the 1st floor prior to coming to the bid opening. Please allow time for this as late submission will not be accepted.

Refer all questions relative to the business aspect, Instructions to Bidders, Special Conditions, and questions concerning the technical aspect of the documents to the Director of Purchasing by email at purchasingdeptstaff@rps205.com.

ROCKFORD BOARD OF EDUCATION

By: Dane Youngblood
Director of Purchasing

March 8, 2022

Mr. Chris Liszka
Construction Project Manager
Rockford Public Schools
5052 28th Ave.
Rockford, IL 61109

Geotechnical Engineering Report
East High School Parking Lot Reconstruction
2929 Charles Street
Rockford, IL 61108
GEOCON Project No. 22-G0339

Dear Mr. Liszka:

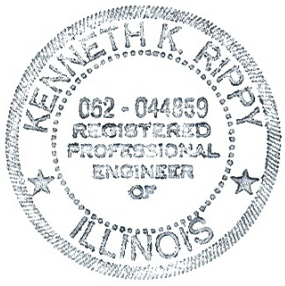
Pursuant to our proposal for geotechnical engineering services, we have completed a subsurface exploration and geotechnical analyses for the above referenced project. This Geotechnical Engineering Report includes our findings and recommendations for the proposed project referenced above.

GEOCON Professional Services, Inc. (GEOCON) appreciates the opportunity to be of service during this phase of the project. If there are any questions or comments you may have regarding the contents of this report, or if we may be of any further service, please contact us at your convenience.

Sincerely,

GEOCON Professional Services, LLC.

Nelson Hatheway
Project Engineer



Kenneth K. Rippy, PE
Senior Geotechnical Engineer

GEOTECHNICAL ENGINEERING REPORT

East High School Parking Lot Reconstruction
2929 Charles Street
Rockford, IL 61108

Mr. Chris Liszka
Construction Project Manager
Rockford Public Schools
5052 28th Ave.
Rockford, IL 61109

Prepared by:
GEOCON Professional Services, LLC.
9370 West Laraway Road, Suite D
Frankfort, Illinois 60423

March 8, 2022

GEOCON Project No. 22-G0339

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GEOTECHNICAL ENGINEERING REPORT
East High School Parking Lot Reconstruction
2929 Charles Street
Rockford, IL 61108

INTRODUCTION

This report presents the results of a subsurface exploration for the proposed parking lot improvements at East High School referenced above. The purpose of this report was to determine and evaluate the subsurface conditions of the existing parking lot and to provide recommendations for subgrade stabilization associated with the parking lot reconstruction.

Authorization to perform this subsurface exploration and analysis was provided in the form of an acceptance of GEOCON Proposal No 22-P062, dated January 27, 2022. The above referenced document described the project scope and contained general conditions for performance of the work.

SITE AND PROJECT DESCRIPTION

The subject site is located at 2929 Charles Street in the City of Rockford, Illinois. It is understood the project will consist of performing improvements to the existing asphalt parking lot located east of the school building. The improvements will include full depth reconstruction of the existing parking lot. It is anticipated that proposed grades will be near existing grades.

The focus of this investigation was to determine the thickness of the existing pavement structure and evaluate subgrade stability.

SUBSURFACE EXPLORATION

The scope of the exploration, including the number, location, and depth of the borings, was determined by the client. A total of three (3) borings were performed for the project and the borings were extended to depths of 7.5 feet below existing grade. During the drilling operations, at each boring location the thickness of asphalt and the underlying aggregate base was recorded. The locations of the borings are shown on the Boring Location Diagram included in the Appendix. The boring locations were marked in the field by GEOCON prior to drilling. After completion of the borings, the holes were backfilled with soil cuttings and patched with like materials as encountered at the boring.

For safety reasons, the boreholes were not allowed to remain open and precluded the collection of delayed water readings.

Drilling and Sampling Procedures

The soil borings were performed with a truck-mounted drill rig equipped with a rotary head. Conventional, continuous flight augers were used to advance the borings with representative samples obtained in each boring employing split-barrel sampling techniques in accordance with ASTM Procedure D-1586. Soil samples were secured at 2.5-foot intervals to a depth of 7.5 feet below grade.

The Standard Penetration Test (SPT) is defined as the number of blows required to advance a 2-inch O.D., split-barrel sampler a distance of one foot by a 140-pound hammer falling 30 inches, commonly described as the N-value. These sampler resistances provide a useful indication of the consistency or relative density of most soil deposits and are reported on the boring logs presented in the Appendix. Samples of cohesive soils obtained from the borings were tested with a calibrated hand penetrometer to aid in evaluating the soil strength characteristics. The results from this testing is tabulated on the boring logs.

Water level observations were made during drilling operations and upon completion and of the borings. This data is noted on the boring logs and within the *Groundwater Conditions* section.

It should be noted that it is difficult to determine the stratigraphy of the upper 2 to 3 feet of the profile from soil borings due to the size of the bore hole, about 6 inches in diameter, and intermittent sample intervals. Further, the split spoon sampler tends to push through softer soils such as fill or topsoil, resulting in little or no sample recovery from these soils. It is recommended that shallow test pits be excavated to better define the exact depth of topsoil or fill if such information is required prior to construction.

Laboratory Tests

Additional characteristics of the foundation materials were determined in the laboratory to provide data on which to classify and estimate the engineering properties of the subsurface soil deposits encountered in the borings. All samples were visually classified by the geotechnical engineer according to the Unified Soil Classification System (ASTM D-2488). An explanation of the symbols used in this system is included in the Appendix.

Representative samples were tested in the laboratory to determine the natural moisture content of the soils. All moisture contents are expressed as a percentage of the dry weight of soil. Representative samples of the cohesive soils encountered in the borings were tested in the laboratory with a calibrated RIMAC spring tester to determine the approximate unconfined compressive strength of the soil samples.

The laboratory testing program selected for this project is intended to assist with determination of soil classification as well as strength and deformation characteristics of the subsurface soil deposits that will provide foundation support for the proposed parking lot. All laboratory testing was performed in general accordance with the respective ASTM Methods, as applicable, and the results are included on the boring logs included in the Appendix. Unless notified to the contrary, all samples will be disposed of after one month.

EXISTING PAVEMENT SECTION

The following tables provides a summary of the pavement sections encountered at the borings in the parking lot measured at the time of drilling.

TABLE 1: EXISTING PAVEMENT SECTION THICKNESS

Boring No.	HMA Thickness (in.)	Aggregate Base Thickness (in.)	Subgrade Type	Subgrade Moisture Content (%)
B-1	2.0	6	CL	26.2
B-2	1.5	7	CF	21.3
B-3	1.5	7	TS	26.3

CL=Lean Clay, CF=Clay Fill, TS= Topsoil

SOIL CONDITIONS

The types of materials encountered at the test boring locations are described on the Soil Boring Logs. The lines delineating the changes in strata on the logs represent an approximate boundary between the various soil classifications. It must be recognized that the soil descriptions are considered representative for the specific test hole location, but the variations may occur between the sampling intervals and boring locations. A summary of the major soil profile components is described in the following paragraphs. A more detailed description and supporting data for each boring location can be found on the individual boring logs.

The surface material consisted of approximately 1.5 to 2 inches of asphalt underlain by approximately 6 to 7 inches of aggregate base.

Below the pavement section, dark grey and brown lean clay was encountered at soil boring B-1 and extended to a depth of about 6 feet below grade. The native lean clay was described as medium stiff with moistures ranging from 23.4 to 26.2 percent. The lean clay was underlain by poorly graded sand extending to termination depth of 7.5 feet below grade and was described as medium dense with a moisture content of 5.7 percent. At soil boring B-2, clay fill was encountered and extended to the predetermined termination depth of 7.5 feet below grade. The clay fill was described as stiff with moistures ranging from 14.3 to 28.2 percent. Soil boring B-3 encountered black clayey topsoil that extended to a depth of about 3.5 feet below grade and was described as stiff with approximately 26.3 percent moisture. The clayey topsoil was underlain by brown lean clay extending to termination depth of 7.5 feet below grade. The lean clay was described as stiff to very stiff with unconfined compressive strengths ranging from 1.2 to 2.2 tsf and moisture contents ranging from 10.6 to 18.8 percent. Organic contents of the clay fill and clayey topsoil ranged from 5.0 to 5.2 percent.

Further information regarding the soil conditions can be found on the boring logs included in the Appendix.

GROUNDWATER CONDITIONS

At the time of drilling, groundwater was not encountered and the bore holes were dry upon completion. It should be noted that due to low permeability soils and the relatively limited stabilization times under which the water levels measurements were made, these observed groundwater level readings may not indicate the water level that may be encountered at the time of construction. In addition, groundwater levels fluctuate over time and are influenced by seasonal precipitation and varying permeability characteristics of the subsurface soils.

ENGINEERING RECOMMENDATIONS

Pavement Subgrade Evaluation and Preparation

It is understood that the existing asphalt parking lot will undergo full depth reconstruction. The existing aggregate base may be considered to be reused as subgrade improvement, but it is not recommended to reuse the existing stone as part of the pavement section design.

It is anticipated that the subgrade in the pavement construction areas may mostly consist of lean clay or clay fill with moisture contents ranging from 21.3 to 26.2 percent. The optimum moisture of this material ranges from 13 to 18 percent, indicating in place subgrade moisture is as much as 8 percent over optimum. Due to the high moisture contents, consideration should be given to including subgrade improvement in the plans for this project. **Lime modification subgrade improvement** would be a viable option for subgrade improvement and would allow mass grading to continue during adverse weather conditions. As another subgrade improvement option, we would recommend stabilizing the subgrade

with a minimum of 12 inches of IDOT CA-6 in areas of clayey topsoil, or where the subgrade is unstable, wet, or the contractor is unable to achieve compaction.

After removing the existing pavement section and prior to placement of new aggregate base, the exposed clay subgrade should be thoroughly tested and evaluated by a representative of the geotechnical engineer. The testing and inspection may consist of proofrolling the subgrade with a heavily loaded single axle dump truck, or DCP testing. DCP testing should be performed in general accordance with the methods and procedures provided in the Illinois Department of Transportation (IDOT) S-33 *"Soils Field Testing and Inspection"* guide.

All earthwork, new embankment construction and subgrade preparation should be in accordance with Division 200 and 300 of the IDOT Standard Specifications. Remedial work for unstable subgrade should consist of disking, aerating, and recompacting exposed subgrade soils, as provided for in Article 301.04 of the IDOT Standard Specifications. Depending upon grading requirements and specific site conditions, solutions to persistent pumping problems may include removal of unstable soils and replacement with IDOT CA-1 crushed limestone aggregate. The CA-1 is typically placed on geogrid in minimum lifts of 1 foot and capped off with a minimum of 6 inches of IDOT CA-6 to reduce surface water to infiltrate the subgrade. The need for undercutting unstable subgrade and replacement fill should be based on direct observations made during construction once the subgrade soils are exposed and density, proof roll, or DCP testing procedures can be conducted.

Pavements should be designed with sufficient slopes to promptly remove surface water. Ponding of water on pavement sections and saturation of pavement subgrades is a common cause of premature pavement deterioration. Routine maintenance consisting of repairing damaged areas is helpful in maintaining pavement life and serviceability. Pavement specifications should reference the Illinois Department of Transportation (IDOT) Standard Specifications.

The subgrade stability will be influenced by such factors as surface drainage provided by the contractor as well as the prevailing temperature and precipitation experienced during construction. The amount of traffic and subgrade disturbance created by heavy construction vehicles will also have an influence on subgrade stability. The contractor should try to make full use of inlets or ditches in order to maintain positive drainage for subgrade areas. Temporary drainage ditches or pumping from depressed areas should be provided as needed during construction in order to prevent ponded water from affecting the stability of the roadway subgrade.

The need for undercutting unstable subgrade and replacement fill should be based on direct observations made during construction once the subgrade soils are exposed and density testing or DCP testing procedures can be conducted. Good construction practice requires testing immediately prior to undercutting the subgrade in order to document the need for the undercut and replacement fill.

GEOCON recommends that the evaluation of the subgrade and selection of fill materials for various applications should be done in consultation with the geotechnical engineer, and placement of fill for structural applications be monitored and tested by a representative of the geotechnical engineer.

CONSTRUCTION CONSIDERATIONS

Groundwater Control

Groundwater and or surface water infiltration may be encountered during mass grading operations, especially if construction occurs during or after periods of increased precipitation. When designing site drainage patterns, site runoff should be diverted away from any structures and directed towards on-site detention areas, or storm sewer systems. Such measures reduce the potential for softening and possible

erosion of the foundation and pavement subgrade soils. It is especially important that water not be allowed to collect next to the building foundations.

Excavations

All excavations should comply with the requirements of OSHA 29CFR, Part 1926, Subpart P, "Excavations," regarding excavation and trench safety, as well as other applicable codes.

GENERAL COMMENTS

This geotechnical exploration and foundation analysis has been conducted to aid in the evaluation of the subsurface conditions on the subject site. The recommendations presented herein are based on the available soil information obtained and the design information provided. Any changes in the soil conditions encountered during construction, design, or building locations should be brought to the attention of the soils engineer to determine if modifications in the recommendations are required. The final design plans and specifications should also be reviewed by the soils engineer to determine that the recommendations presented herein have been interpreted and implemented as intended. It is recommended that the earthwork and foundation operations be monitored by the Geotechnical Engineer, to test and evaluate the bearing capacities, and the selection, placement, and compaction of controlled fills.

This geotechnical study has been conducted in a manner consistent with that level of care ordinarily exercised by members of the profession currently practicing in the same locality under similar conditions. The findings, recommendations, and opinions contained herein have been promulgated in accordance with generally accepted practice in the fields of foundation engineering, soils mechanics, and engineering geology. No other representations expressed or implied, and no warranty or guarantee is included or intended in this report.

APPENDIX

Boring Location Diagram
Soil Boring Logs
General Notes

GEOCON PROFESSIONAL SERVICES
9370 Laraway Road, Suite D
Frankfort, IL 60423
P.815.806.9986 F. 815.464.8691

FIGURE 1
SITE VICINITY MAP
East High School – Parking Lot
2929 Charles Street
Rockford, IL 61108

PROJECT NUMBER: 22-G0339

DATE: March 2022

GEOCON PROFESSIONAL SERVICES
9370 Laraway Road, Suite D
Frankfort, IL 60423
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FIGURE 2
BORING LOCATION DIAGRAM
East High School – Parking Lot
2929 Charles Street
Rockford, IL 61108

PROJECT NUMBER: 22-G0339

DATE: March 2022

BORING NO. B-1

GEOCON PROFESSIONAL SERVICES

CLIENT Rockford Public Schools **PROJECT NAME** East High School Parking Lot Replacement

PROJECT NUMBER 22-G0339 **PROJECT LOCATION** 2929 Charles St., Rockford, IL

DATE COMPLETED 2/17/22 **LOGGED BY** JC/MR **DRILLING METHOD** 4 in. Solid Stem Auger

DEPTH (ft)	ELEVATION (ft.)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (Qp) (tsf)	UNC. STRENGTH (Qu) (tsf)	MOISTURE CONTENT (%)	DRY UNIT WT. (pcf)	ORGANIC CONTENT (%)	ATTERBERG LIMITS: LIQUID LIMIT	ATTERBERG LIMITS: PLASTIC LIMIT	ATTERBERG LIMITS: PLASTICITY INDEX
0.0	780.8		2" ASPHALT											
	780.3		6" AGGREGATE BASE											
			dark grey and brown LEAN CLAY medium stiff	SS 1	100	3-4-4 (8)	0.75		26.2					
2.5														
	777.0		brown LEAN CLAY w/ GRAVEL AND SAND medium stiff	SS 2	100	3-9-7 (16)	0.5		23.4					
5.0														
	775.0		brown POORLY GRADED SAND w/ GRAVEL medium dense	SS 3	89	4-5-6 (11)			5.7					
7.5	773.5													

Bottom of borehole at 7.5 feet.

COMPLETION DEPTH 7.5 ft **GROUND ELEVATION** 781 ft

CAVE DEPTH ft **BACKFILL** Soil Cuttings

GROUND WATER LEVELS:

AT TIME OF DRILLING --- None

AT END OF DRILLING --- Dry Upon Completion

AFTER DRILLING ---

NOTES

Groundwater levels were recorded at the time of drilling and may not represent the groundwater conditions at the time of construction.

Lines of Demarcation represent an **approximate** boundary between soil types. Variations may occur between sampling intervals and between boring locations, and the transition may be gradual. Dashed lines are indicative of potentially erratic or unknown changes.

BORING NO. B-2

GEOCON PROFESSIONAL SERVICES

CLIENT Rockford Public Schools **PROJECT NAME** East High School Parking Lot Replacement

PROJECT NUMBER 22-G0339 **PROJECT LOCATION** 2929 Charles St., Rockford, IL

DATE COMPLETED 2/17/22 **LOGGED BY** JC/MR **DRILLING METHOD** 4 in. Solid Stem Auger

DEPTH (ft)	ELEVATION (ft.)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (Qp) (tsf)	UNC. STRENGTH (Qu) (tsf)	MOISTURE CONTENT (%)	DRY UNIT WT. (pcf)	ORGANIC CONTENT (%)	ATTERBERG LIMITS: LIQUID LIMIT	ATTERBERG LIMITS: PLASTIC LIMIT	ATTERBERG LIMITS: PLASTICITY INDEX
0.0	778.9		1.5" ASPHALT											
	778.3		7" AGGREGATE BASE											
			MIXED CLAY w/ TOPSOIL (FILL) trace gravel stiff	SS 1	100	12-7-8 (15)	1.5		21.3					
2.5														
				SS 2	56	3-2-2 (4)	1.5		28.2		5.0			
5.0														
				SS 3	89	2-1-3 (4)	1.0		14.3					
7.5	771.5													

Bottom of borehole at 7.5 feet.

COMPLETION DEPTH 7.5 ft **GROUND ELEVATION** 779 ft

CAVE DEPTH ft **BACKFILL** Soil Cuttings

GROUND WATER LEVELS:

AT TIME OF DRILLING --- None

AT END OF DRILLING --- Dry Upon Completion

AFTER DRILLING ---

NOTES

Groundwater levels were recorded at the time of drilling and may not represent the groundwater conditions at the time of construction.

Lines of Demarcation represent an **approximate** boundary between soil types. Variations may occur between sampling intervals and between boring locations, and the transition may be gradual. Dashed lines are indicative of potentially erratic or unknown changes.

BORING NO. B-3

GEOCON PROFESSIONAL SERVICES

CLIENT Rockford Public Schools **PROJECT NAME** East High School Parking Lot Replacement

PROJECT NUMBER 22-G0339 **PROJECT LOCATION** 2929 Charles St., Rockford, IL

DATE COMPLETED 2/17/22 **LOGGED BY** JC/MR **DRILLING METHOD** 4 in. Solid Stem Auger

DEPTH (ft)	ELEVATION (ft.)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	POCKET PEN. (Qp) (tsf)	UNC. STRENGTH (Qu) (tsf)	MOISTURE CONTENT (%)	DRY UNIT WT. (pcf)	ORGANIC CONTENT (%)	ATTERBERG LIMITS: LIQUID LIMIT	ATTERBERG LIMITS: PLASTIC LIMIT	ATTERBERG LIMITS: PLASTICITY INDEX
0.0	782.8		1.5" ASPHALT											
	782.4		7" AGGREGATE BASE											
			black CLAYEY TOPSOIL stiff	SS 1	100	7-7-5 (12)	1.5		26.3		5.2			
2.5	779.0		brown LEAN CLAY trace sand stiff to very stiff	SS 2	100	4-5-3 (8)	1.25	1.2	18.8					
5.0				SS 3	89	3-4-4 (8)	2.25	2.2	10.6					
7.5	775.5		Bottom of borehole at 7.5 feet.											

COMPLETION DEPTH 7.5 ft **GROUND ELEVATION** 783 ft

CAVE DEPTH ft **BACKFILL** Soil Cuttings

GROUND WATER LEVELS:

AT TIME OF DRILLING --- None

AT END OF DRILLING --- Dry Upon Completion

AFTER DRILLING ---

NOTES

Groundwater levels were recorded at the time of drilling and may not represent the groundwater conditions at the time of construction.

Lines of Demarcation represent an **approximate** boundary between soil types. Variations may occur between sampling intervals and between boring locations, and the transition may be gradual. Dashed lines are indicative of potentially erratic or unknown changes.

GENERAL NOTES

CLIENT Rockford Public Schools **PROJECT NAME** East High School Parking Lot Replacement

PROJECT NUMBER 22-G0339 **PROJECT LOCATION** 2929 Charles St., Rockford, IL

SAMPLE IDENTIFICATION

Visual soil classifications are made in general accordance with the United Soil Classification System (USCS) on the basis of textural and particle size categorization, and various soil behavior characteristics. Visual classifications should be substantiated by appropriate laboratory testing when a more exact soil identification is required to satisfy specific project applications criteria.

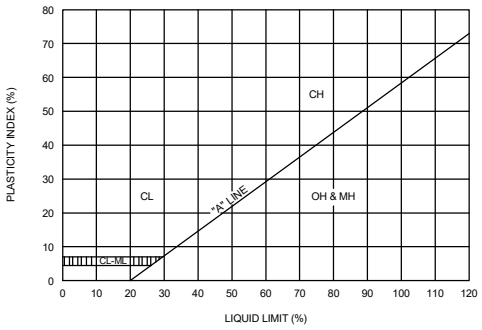
UNIFIED SOIL CLASSIFICATION SYSTEM (ASTM D-2487-98)

MATERIAL TYPES	CRITERIA FOR ASSIGNING SOIL GROUP NAMES			GROUP SYMBOL	SOIL GROUP NAMES & LEGEND
COARSE-GRAINED SOILS >50% RETAINED ON NO. 200 SIEVE	GRAVELS >50% OF COARSE FRACTION RETAINED ON NO 4. SIEVE	CLEAN GRAVELS <5% FINES	$C_u \geq 4$ AND $1 \leq C_c \leq 3$	GW	WELL-GRADED GRAVEL
			$C_u \geq 4$ AND/OR $1 \geq C_c \geq 3$	GP	POORLY-GRADED GRAVEL
		GRAVELS WITH FINES >12% FINES	FINES CLASSIFY AS ML OR CL	GM	SILTY GRAVEL
			FINES CLASSIFY AS CL OR CH	GC	CLAYEY GRAVEL
	SANDS >50% OF COARSE FRACTION PASSES ON NO 4. SIEVE	CLEAN SANDS <5% FINES	$C_u \geq 6$ AND $1 \leq C_c \leq 3$	SW	WELL-GRADED SAND
			$C_u \geq 6$ AND/OR $1 \geq Cc \geq 3$	SP	POORLY-GRADED SAND
		SANDS AND FINES >12% FINES	FINES CLASSIFY AS ML OR MH	SM	SILTY SAND
			FINES CLASSIFY AS CL OR CH	SC	CLAYEY SAND
FINE-GRAINED SOILS >50% PASSES NO. 200 SIEVE	SILTS AND CLAYS LIQUID LIMIT<50	INORGANIC	PI>7 AND PLOTS>"A" LINE	CL	LEAN CLAY
			PI>4 AND PLOTS<"A" LINE	ML	SILT
		ORGANIC	LL (oven dried)/LL (not dried)<0.75	OL	ORGANIC CLAY OR SILT
	SILTS AND CLAYS LIQUID LIMIT>50	INORGANIC	PI PLOTS >"A" LINE	CH	FAT CLAY
			PI PLOTS <"A" LINE	MH	ELASTIC SILT
		ORGANIC	LL (oven dried)/LL (not dried)<0.75	OH	ORGANIC CLAY OR SILT
HIGHLY ORGANIC SOILS		PRIMARILY ORGANIC MATTER, DARK IN COLOR, AND ORGANIC ODOR		PT	PEAT

PROJECT LITHOLOGIC SYMBOLS (USCS)

- ASPHALT: Asphalt
- FILL: Fill (made ground)
- TOPSOIL: Topsoil
- CL: USCS Low Plasticity Clay
- GRAVEL FILL: Gravel Fill
- CLS: USCS Low Plasticity Sandy Clay
- SP: USCS Poorly-graded Sand

PLASTICITY CHART



PROJECT SAMPLE TYPES

- Split Spoon (SS)

SOIL RELATIVE DENSITY AND CONSISTENCY CLASSIFICATION

NON-COHESIVE SOILS		COHESIVE SOILS		
RELATIVE DENSITY	N-VALUE*	CONSISTENCY	N-VALUE*	COMPRESSIVE STRENGTH (TSF)
VERY LOOSE	0 - 4	VERY SOFT	0 - 2	0 - 0.25
LOOSE	4 - 10	SOFT	2 - 5	0.25 - 0.50
MEDIUM DENSE	10 - 30	MEDIUM STIFF	5 - 10	0.50 - 1.0
DENSE	30 - 50	STIFF	10 - 14	1.0 - 2.0
VERY DENSE	OVER 50	VERY STIFF	14 - 32	2.0 - 4.0
		HARD	OVER 32	OVER 4.0

* N-VALUE: NUMBER OF BLOWS OF 140 LB HAMMER FALLING 30 INCHES TO DRIVE A 2 INCH O.D. (1-3/8 INCH I.D.) SPLIT-BARREL SAMPLER THE LAST 12 INCHES OF AN 18-INCH DRIVE (ASTM-1586 STANDARD PENETRATION TEST).

ABBREVIATIONS

SS	-	SPLIT-SPOON SAMPLE	LL	-	LIQUID LIMIT (%)
ST	-	SHELBY TUBE SAMPLE	PL	-	PLASTIC LIMT (%)
AU	-	AUGER SAMPLE	PI	-	PLASTIC INDEX (%)
MC	-	MOISTURE CONTENT (%)	NP	-	NON PLASTIC
-200	-	PERCENT PASSING NO. 200 SIEVE	DD	-	DRY DENSITY (PCF)
Qp	-	POCKET PENETROMETER (TSF)	DCP	-	DYNAMIC CONE PENETROMETER
Qu	-	UNCONFINED STRENGTH (TSF)	IBV	-	IMMEDIATE BEARING VALUE

9370 W. Laraway Road, Suite D Frankfort, IL 60423 Phone 815-806-9986 Fax 815-464-8691

ROCKFORD PUBLIC SCHOOLS
IFB No. 22-40 East H.S. Student Parking
Lot Improvements
Pre-Bid Meeting Sign-In Sheet
April 13, 2022 at 10:00 a.m.

ROCKFORD PUBLIC SCHOOLS
WORLD-CLASS EDUCATION FOR ALL CHILDREN

PRE-BID MEETING SIGN-IN SHEET

IFB No. 22-40 East H.S. Parking Lot Improvements

PLEASE WRITE EMAIL ADDRESS SO THAT IT IS LEGIBLE IN ORDER TO RECEIVE ADDENDUM INFORMATION

	Printed Name	Company Name	Company Address	Telephone	E-mail
1	*Chris J. Liszka*				
2	*RPS205 Construction Project Manager* *5052 28th Ave., Rockford IL 61109* *P: 815-378-3632* *christopher.liszka@rps205.com*				
3	Scott Johnson	Herbig Blacktop	900 Depot Ave Dixon IL 61021	815-677-5583	SJohnson@herbigblacktop.com
4	BRIAN AMLING	NORTHERN ILLINOIS SERVICE CO.	4761 SAWY HOLLOW RD. ROCKFORD 61109	815.874.4422	ESTIMATING@NORTHERNILLINOISSERVICES.COM
5	Seth Gronewold	Fehr Graham	200 Prairie St. Suite 208 Rockford, IL 61107	815-394-4700	sgronewold@fehrgraham.com
6	Kevin Kercinar	Stenstrom	2422 Center St Rockford, IL 61108	815-308-3478	Excavation@rstenstrom.com
7	Greg Shelton	Rock Road	PO Box 1818 Janesville, WI 53547	608-752-8944	bids@rockroads.com

ROCKFORD PUBLIC SCHOOLS
IFB No. 22-40 East H.S. Student Parking Lot Improvements
Pre-Bid Meeting Sign-In Sheet
April 13, 2022 at 10:00 a.m.

	Printed Name	Company Name	Company Address	Telephone	E-mail
8					
9	Alberto Landeros	Antigua inc	3604 N Laberge Ave Chicago IL	7084905311	landeros@Antiguaec.com
10	Curt Szymanski	NTRAK	1523 WINDSOR RD. LOMBARD, IL 60148	8155893743	CURT@NTRAKGROUP.com
11	JOHN HALLING	WILLIAM CHARLES	833 FEATHERSTONE ROCKFORD, IL 61107	815-298-3633	JOHN.HALLING@WILLIAMCHARLES.com
12					
13					
14					
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ROCKFORD PUBLIC SCHOOLS
IFB No. 22-40 East H.S. Student Parking
Lot Improvements
Pre-Bid Meeting Sign-In Sheet
April 13, 2022 at 10:00 a.m.

PRE-BID CONFERENCE OPENING STATEMENT

Welcome to the mandatory pre-bid conference for IFB No. 22-40 East H.S. Parking Lot Improvements for the Rockford Public Schools.

The purpose of this meeting is to receive input, comments, questions, clarifications and suggested changes relative to this solicitation. As a reminder, the only acceptable changes to the Bid/RFP are formal Addenda published by the RPS Purchasing department. Additionally, the Addendum may address other issues identified by the School District.

The goal of today's meeting is to increase your knowledge of the solicitation as it is written and provide an information mechanism in which you may advise the School District of any changes it should make. Consequently, any changes you wish the Rockford Public Schools to consider must be submitted in writing to the Purchasing department before the deadline as expressed in the solicitation.

We will try to answer as many of the questions as possible. If we cannot answer a question today, we will defer that answer to the published Addendum. Additionally, minutes from this pre-bid conference will be published in the Addendum.

- Bid Opening is scheduled for April 28, 2022 at 2:00 pm Rockford Board of Education, 6th floor Conference Room. Late bids will not be accepted. Faxed or emailed bids will not be accepted.

- Board Approval May 10, 2022.

- Bid RFI Procedures - All written correspondence during the bid process MUST be sent to Dane Youngblood, Director of Purchasing, via email at PurchasingDeptStaff@rps205.com. Last RFI will be accepted until April 19, 2022 at 12 pm. Last addendum will be issued by April 21, 2022 at 4:30 pm.

- Addenda will be emailed to all attendees at the pre-bid conference, posted on the RPS website and Bonfire.

- PLEASE reference the **REQUIRED FORMS CHECK LIST** for all documents that **must** be submitted with your bid offer form. All forms must be properly completed, signed and submitted or your bid will be deemed non-responsive.